

# Equation Sheet

## Formulas

### Hooke's law:

$$\varepsilon = (L_f - L_i)/L_i$$

$$\varepsilon_x = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)] + \alpha \Delta T$$

$$\varepsilon_y = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)] + \alpha \Delta T$$

$$\varepsilon_z = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)] + \alpha \Delta T$$

$$\gamma_{xy} = \frac{1}{G} \tau_{xy} \quad \gamma_{xz} = \frac{1}{G} \tau_{xz} \quad \gamma_{yz} = \frac{1}{G} \tau_{yz}$$

$$\sigma_x = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\varepsilon_x + \nu(\varepsilon_y + \varepsilon_z)]$$

$$\sigma_y = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\varepsilon_y + \nu(\varepsilon_z + \varepsilon_x)]$$

$$\sigma_z = \frac{E}{(1+\nu)(1-2\nu)} [(1-\nu)\varepsilon_z + \nu(\varepsilon_x + \varepsilon_y)]$$

### Axial deformation, thermal expansion:

$$e = \frac{FL}{EA} + L\alpha\Delta T$$

$$F = \frac{EA}{L}(e - L\alpha\Delta T)$$

$$F = K(e - L\alpha\Delta T)$$

$$e = u \cos(\theta) + v \sin(\theta)$$

### Torsion:

$$\tau = G r \frac{\phi}{L}$$

$$\tau = \frac{Tr}{I_p}$$

$$\phi = \frac{TL}{GI_p}$$

$$K = \frac{GI_p}{L}$$

$$T = K\phi$$

### Stress transformation and Mohr's circle:

$$I_{p\_Circular\_Cross\_Section} = \frac{\pi d^4}{32}$$

$$\sigma_{x'} = \left( \frac{\sigma_x + \sigma_y}{2} \right) + \left( \frac{\sigma_x - \sigma_y}{2} \right) \cos 2\theta_{xx'} + \tau_{xy} \sin 2\theta_{xy}$$

$$I_{p\_Hollow\_Circular\_Cross\_Section} = \frac{\pi(d_o^4 - d_i^4)}{32}$$

$$\sigma_{y'} = \left( \frac{\sigma_x + \sigma_y}{2} \right) - \left( \frac{\sigma_x - \sigma_y}{2} \right) \cos 2\theta_{xx'} - \tau_{xy} \sin 2\theta_{xy}$$

$$\tau_{x'y'} = - \left( \frac{\sigma_x - \sigma_y}{2} \right) \sin 2\theta_{xx'} + \tau_{xy} \cos 2\theta_{xy}$$

### Stress due to bending moment:

$$\sigma(x, y) = \frac{-E(x)y}{\rho(x)} = \frac{-M(x)y}{I_{zz}}$$

$$\sigma_{p1} = \sigma_{avg} + R$$

$$\sigma_{p2} = \sigma_{avg} - R$$

$$I_{zz} = \frac{bh^3}{12} \text{ (rectangle)} \quad I_{zz} = \pi \frac{d^4}{64} \text{ (circle)}$$

$$\tau_{max} = R$$

### Stress due to shear force:

$$\sigma_{avg} = \left( \frac{\sigma_x + \sigma_y}{2} \right)$$

$$\tau(x, y) = \frac{V(x)Q(y)}{I_z b} \quad Q(y) = \int_{A'} \eta dA = A' \bar{y}'$$

$$R = \sqrt{\left( \frac{\sigma_x - \sigma_y}{2} \right)^2 + \tau_{xy}^2}$$

$$(\tau_{max})_{N.A.} = \frac{3V}{2A} \text{ (rectangle)} \quad (\tau_{max})_{N.A.} = \frac{4V}{3A} \text{ (circle)}$$

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## Failure criteria, factor of safety:

von Mises Equivalent Stress:

$$\sigma_M = \frac{1}{\sqrt{2}} \sqrt{\left(\sigma_{p_1} - \sigma_{p_2}\right)^2 + \left(\sigma_{p_2} - \sigma_{p_3}\right)^2 + \left(\sigma_{p_3} - \sigma_{p_1}\right)^2}$$

von Mises Stress (Plane Stress):

$$\sigma_M = \sqrt{\sigma_{p_1}^2 - \sigma_{p_1}\sigma_{p_2} + \sigma_{p_2}^2}$$

$$FS = \frac{\text{Failure Stress}}{\text{Allowable Stress}}, \frac{\text{Yield Strength}}{\text{State of Stress}}$$

## Buckling:

Critical buckling load for a pinned-pinned beam

$$P_{cr} = EI \frac{\pi^2}{L^2}$$

Critical buckling for fixed-fixed beam

$$P_{cr} = EI \frac{\pi^2}{(0.5L)^2}$$

## Energy methods:

Strain energy:  $U_{tot} = \int \frac{M^2}{2EI} dx + \int \frac{F^2}{2AE} dx + \int \frac{T^2}{2GI_p} dx + \int \frac{f_s V^2}{2AG} dx$  where  $f_s$  is the shape factor

- Castigliano's theorem for deflection  $\Delta$  at a point in the direction of the force  $\Delta = \frac{\partial U_{tot}}{\partial P}$  where  $P$  is the point force
- Castigliano's theorem for slope  $\theta_c$  or angle of twist  $\phi_c$  in the direction of an applied moment  $M$  or applied torque  $T$  is given by  $\theta_c = \frac{\partial U_{tot}}{\partial M}$  or  $\phi_c = \frac{\partial U_{tot}}{\partial T}$
- Work-energy theorem states that the deflection  $\Delta$  at the point of application of a force  $P$  in the direction of applied force can be calculated by equating  $\frac{1}{2}P\Delta = U_{tot}$
- Work-energy theorem states that the slope  $\theta_c$  or angle of twist  $\phi_c$  in the direction of an applied moment  $M$  or applied torque  $T$  or can be calculated by equating  $\frac{1}{2}M\theta_c = U_{tot}$  or  $\frac{1}{2}T\phi_c = U_{tot}$

## Stress in pressure vessels:

$$\sigma_{spherical} = \frac{pr}{2t}; \quad \sigma_h = \frac{pr}{t}; \quad \sigma_a = \frac{pr}{2t}$$

2<sup>nd</sup> order equation to solve for deflection curve

$$EI \frac{d^2v(x)}{dx^2} = M(x)$$